

Application for

Planning Commission Action

Community Development Department Planning Division



OFFICE USE ONLY Date Received:

Complete this application in its entirety along with any required Supplements to the address below:

City of Hampton Community Development Department, Planning Division 22 Lincoln Street, 5th Floor Hampton, Virginia 23669

Case Number: PCA	 	 		

1. PROPERTY INFORMATION		
Address or Location Parcel C Pun	np Station	
LRSN 2000401	Zoning District	DT1
Current Land Use Vacant		
Proposed Land Use New Pump S	tation 107	
2. SITE INFORMATION		
Total Number of Structures Proposed 1	Freestanding 1	Enclosed1
Maximum Height of all Structures 18	Net Square Foota	ge of all Structures 1000
Site Acreage34 ac	Proposed Number of On-	site Parking Spaces0
3. PROPERTY OWNER INFORMATION (a	an individual or a legal entity m	ay be listed as owner)
Owner's Name HRHA		
Address 1 Franklin Street Ste 603	_{City} Hampton	_{State} VA _{Zip} 23669
Phone 7578793566		
4. APPLICANT INFORMATION (if differen	•	
Applicant's Name Trilogy Develor	pers, LLC	
Address 1439 N. Great Neck Road	_{City} Virginia Beac	h _{State} VA _{Zip} 23454
Phone 7574070069		
5. APPLICANT AGENT INFORMATION (if	f different from applicant)	
Agent's Name		
Address	City	State Zip
Phone	Email	

6. CERTIFICATION FOR LEGAL ENTITY PROPERTY OWNERS

HRHA

Complete this section only if the property owner is **not** an individual but rather a legal entity such as a corporation, trust, LLC, partnership, diocese, etc. as specified in Step 2 above.

"I hereby submit that I am legally authorized to execute this application on behalf of the fee-simple owner of this property. I have read this application and it is submitted with my full knowledge and consent. I authorize city staff and representatives to have access to this property for inspection. The information contained in this application is accurate and correct to the best of my knowledge."

Name(s), title(s), signature(s), and date(s) of authorized representative(s) of the legal entity (attach additional page if necessary):

Name of	Legal Entity	<u> </u>	
Signed by	y: Name (printed) Aa	ru Ma`at	, Its (title) Executive Director
	Signature	euract-	Date
	Name (printed)		, Its (title)
	Signature		Date
	Name (printed)		, its (title)
	Signature		Date
my full kn inspection Name(s),	owledge and consent. I and consent. I and contain the information contain and date(s	authorize city staff and represent	
			Date
Name (pr	rinted)		
Signature	•		Date
Γ		OFFICE USE ONLY	
	☐ Application Form	☐ Narrative Statement	☐ Supplemental Form (if required)
	☐ Application Fee	☐ Survey Plat	☐ Additional materials (if required)

Design Narrative

for

City of Hampton Pump Station #107 Replacement

Hampton, Virginia September 13, 2024

Prepared for:

Trilogy Developers, LLC. 1439 N Great Neck Road Virginia Beach, VA 23454





2901 S. Lynnhaven Road, Suite 200 Virginia Beach, Virginia 23452 P: 757-213-6679

F: 757-340-1415

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Project Description

This project is located along Michigan Drive in Hampton, Virginia. This project proposes the demolition and replacement of an existing pump station with a new pump station. The total land disturbance is 0.25 acres, with 0.16 acres occurring in the right-of-way, 0.06 acres occurring on the new proposed site, and 0.03 acres occurring on the site of the existing pump station that will be demolished and replaced. The site also proposes a drive aisle, a generator pad, and a transformer pad. Immediately adjacent to the project site is a Virginia electric and power easement.

Existing Conditions

There are two sites for this project, an existing pump station site and a proposed pump station site.

The existing site is mostly pervious cover with one brick pump station building. The site has managed turf land cover and includes underground sanitary sewer pipes that serve the existing pump station.

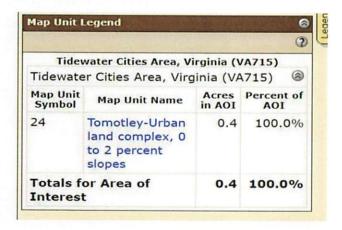
The proposed site is all pervious cover, containing of all managed turf with some trees located around the property line.

Adjacent areas

The site is bounded by Michigan Drive to the south, The Grand Hampton at Langley to the north/northeast, and a Virginia power and electric easement to the west. The project is in close proximity to the Hampton Roads Beltway.

Soil properties

According to the "Web Soil Survey" provided by United States Department of Agriculture (USDA) (http://websoilsurvey.nrcs.usda.gov/app/HomePage.htm) the existing site has the following soil characteristics:



The USDA NRCS Web Soil Survey Maps can be found in Appendix A.

Critical Areas

There are no critical areas within the limits of disturbance. This project is outside of any CBPA or other wetland buffers.

Demolition

Demolition for this project will consist of the destruction of the existing pump station and all infrastructure that serves this pump station (gravity and force main sanitary sewer lines and valves). See the "Sequencing" section for the sequence of demolition and construction for this project.

Sequencing

- 1. Acquire permits and attend presubmittal meeting
- 2. Construct erosion and sediment control measures
- 3. Construct new pump station
- 4. Connect the new pump station to the force main and gravity sewer lines with An open cut of the force main to install a wye and gate valve in accordance with the plan on Sheet C5.0
- 5. Test new pump station and perform startup for city acceptance
- 6. Activate new pump station
- 7. Install new, revised sanitary sewer connections in Michigan Dr to reroute flow from the existing pump station to the new one, as shown in the plans. Contractor to maintain sewer flows to existing pump station until redirection to the new operational pump station is available. Stage construction to minimize bypass pumping. The water line will connect to the existing 8" line with a 1" corporation stop and flow across Michigan Dr perpendicular to the new pump station, and a 5/8" water meter will be installed at the property line.
- 8. Once sanitary flows are diverted to the new pump station, close new gate valve at the wye connection taking the old pump station force main out of service. The existing lines that are no longer in use and the existing pump station will be demolished or abandoned in place.

Site Layout and Access

Access to the site will be provided by one 36' X 20' concrete drive aisle which connects to Michigan Drive.

The site is zoned DT-1, which has zero building setbacks.

Utilities

Domestic Water

The domestic water line for the proposed pump station will come from the existing 8" waterline that is located on the other side of Michigan Dr along the right-of-way line. The proposed waterline will connect to the existing line with a 1" corporation stop. The

proposed line will be a 1" ductile iron pipe that comes straight across Michigan Dr perpendicular to the pump station building. The line will include a 5/8" water meter on the property line and the waterline will connect to the pump station 2 ft from the corner of the building closest to the drive aisle. After the water meter, there will be a tee off for a freeze proof yard hydrant.

Water meter calculations have been provided in Appendix B.

Sanitary Sewer

Sanitary sewer will connect the existing manholes that serve the existing pump station with a new gravity sewer line. This line will run northeast down the middle of Michigan Dr over to the right side of the proposed drive aisle, which it will run northwest and then southwest over to the influent pipe in the proposed pump station. This gravity sewer will be 12" C900 PVC pipe that runs at a 0.3% slope.

A force main line will then leave the proposed pump station to the northeast under the proposed drive aisle. After the pipe crosses the drive aisle, there will be an air release valve, gate valve, then a tee to an emergency pump connection with a gate valve, then the flow meter vault. The force main will have two 45° bends to turn towards Michigan Dr to connect to the existing force main line. There will be a gate valve, then a 45° bend into a wye with another gate valve. This force main line will 8" ductile iron pipe.

Sanitary sewer calculations can be found in Appendix C. We are currently working in cooperation with David Powell, with Hampton Public Works, for pump station operations and detailed design.

Stormwater Considerations

Existing Drainage Patterns

The existing site's drainage flows into the existing access road and parking lot for The Grand Hampton at Langley. This directs water into the inlets within the parking lot and within Michigan Dr. these inlets then convey stormwater into a creek on the other side of the Grand Hampton access road. This creek then moves the water north until it reaches the Southwest Branch Back River.

The USDA "Web Soil Survey" for the project site classifies the soils as Hydrologic Soil Group (HSG) D and urban land. See Appendix A for the soil map.

Stormwater Overview

Most of the land disturbance that will take place for this project will occur in the right-of-way. This will include excavation to demolish and install force and gravity sanitary sewer as well as domestic water. This land disturbance will not impact the existing condition of the right-of-way, the work will excavate the area and then reinstall the same as existing; therefore, drainage patterns and impervious surfaces will not change. Additionally, only 2,490 SF of land disturbance will take place on the new proposed pump station site and only 1,091 SF of land disturbance will take place on the existing pump station site. Both

land disturbance activities remain under the 2,500 SF, which is the maximum allowable land disturbance before stormwater quantity and quality must be taken into consideration. For this reason, no BMP or additional stormwater infrastructure will be needed for this project, as the existing drainage patterns can still accommodate the site. There will not be a dramatic increase in impervious surfaces and the proposed work will continue to have the same drainage patterns as the existing conditions.

Erosion and Sediment Control

Erosion and Sediment Control Measures

Unless otherwise indicated, all vegetative and structural erosion and sediment control practices shall be constructed and maintained in accordance with the minimum standards and specifications of the Virginia Stormwater Management Handbook (VSMH). The minimum standards of the VSMH Regulations shall be adhered to unless otherwise waived or approved by a variance.

The following measures shall be installed and maintained as shown on the erosion & sediment control plans:

Temporary Construction Entrance - std. & spec. C-SCM-03

A temporary stone construction entrance shall be installed according to the minimum standards and specifications set forth in the VSMH.

Silt Fence - std. & spec. C-PCM-04

Temporary silt fence sediment barriers will be installed around the perimeter of the limits of disturbance to prevent sediment-laden runoff from leaving the site. In areas currently occupied by asphalt pavement, silt fence shall be installed immediately after the pavement has been removed.

Topsoiling (Soil Stockpile) - std. & spec. C-SSM-02

Topsoil will be stripped from areas to be graded and stockpiled in such a manner that natural drainage is not obstructed, and no off-site sediment damage shall result. Stabilize or protect stockpiles in accordance with MS-2 of the VSMH. Side slopes of the stockpile shall not exceed 2H:1V. Perimeter controls must be placed around the stockpile immediately. Stockpiles shall be seeded within seven (7) days of the formation of the stockpile, if it is to remain dormant for longer than 14 days.

Temporary Seeding - std. & spec. C-SSM-09

All denuded areas that will remain dormant for a period of time greater than 14 days shall be seeded with fast germinating temporary vegetation immediately following grading. Selection of vegetation will be dependent on the time of year it is applied.

Permanent Seeding - std. & spec. C-SSM-10

Permanent seeding will be established on all non-paved disturbed areas.

Mulching - std. & spec. C-SSM-11

Mulch will be applied to all seeded areas to prevent erosion and foster the growth of vegetation.

Soil Stabilization Blankets and Matting - std. & spec. C-SSM-05

Soil Stabilization Blankets and Matting are proposed to aid in controlling erosion by providing a microclimate that protects young vegetation and promotes its establishment.

Dust Control - std. & spec. C-SCM-01

Areas subject to surface and air movement of dust shall be stabilized during construction to minimize dust release. Methods include but are not limited to vegetative cover, mulch, or irrigation.

Specifications For ESC Measures

Specifications for all E&S measures used on this project can be found in the" Virginia Stormwater Management" (VSMH), as well as on the plan sheets. The contractor is responsible for following all construction specifications, installation procedures, and maintenance procedures as listed in VSMH.

E&S Management Strategies

The following sequence of events and erosion control measures shall be incorporated into the construction schedule for this project and shall apply to all construction activities within the project limits.

- 1. Soil Stabilization:
 - a. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site.
 - b. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days, but less than one year.
 - c. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.
- 2. Soil Stockpile Stabilization: During construction, soil stockpiles and borrow areas shall be stabilized or protected with sediment trapping measures. Temporary protection and permanent stabilization shall be applied to all soil stockpiles on site and borrow areas or soil intentionally transferred off site.
- 3. Permanent Stabilization: Permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved that is:
 - Uniform
 - Mature enough to survive
 - Will inhibit erosion
- 4. Sediment Traps: Sediment traps intended to trap sediment shall be constructed as a first step in any land-disturbing activity and shall be made functional before upslope land disturbance takes place.
- 5. Cut and Fill Slopes Design & Construction: Cut and fill slopes shall be designed and constructed in a manner that will minimize erosion. Slopes found to be eroding excessively within one year of permanent stabilization shall be provided with additional slope stabilizing measures until the problem is corrected.

- 6. Concentrated Runoff Down Slopes: Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume, or slope drain structure.
- 7. Slope Maintenance: Whenever water seeps from a slope face, adequate drainage or other protection shall be provided.
- 8. Storm Sewer Inlet Protection: All storm sewer inlets made operable during construction shall be protected so that sediment-laden water cannot enter the stormwater conveyance system without first being filtered/treated to remove sediment.
- 9. Stormwater Conveyance Protection: Before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and the receiving channel.
- 10. Underground Utility Line Installation: Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria:
 - a. No more than 500 linear feet of trench may be opened at one time
 - b. Excavated material shall be placed on the uphill side of trenches
 - c. Effluent from dewatering operations shall be filtered or passed through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams or off-site property
 - d. Material used for backfilling trenches shall be properly compacted in order to minimize erosion and promote stabilization
 - e. Restabilization shall be accomplished in accordance with these regulations
 - f. Comply with applicable safety regulations
- 11. Vehicular Sediment Tracking: Where construction vehicle access routes intersect paved or public roads:
 - a. Provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface
 - b. Where sediment is transported onto a paved or public road surface, the road surface shall be cleaned thoroughly at the end of each day
 - c. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner
 - 12. Removal of Temporary Measures: All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, unless otherwise authorized by the program authority. Trapped sediment and the disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent further erosion and sedimentation.

E&S Maintenance

In general, all erosion and sediment control measures shall be checked after each rainfall or weekly, whichever is most frequent, and should be cleaned and repaired according to the following schedule.

1. Construction entrance shall be maintained in a condition which will prevent tracking or flow of mud onto paved surfaces and public rights-of-way.

- Maintain construction entrances in accordance with Std. & Spec. C-SCM-03 of the VSMH.
- 2. Silt fences shall be inspected after each rainfall and repaired immediately, as required. Maintain silt fence in accordance with Std. & Spec. C-PCM-04 of the VSMH.
- 3. Erosion and sediment control measures shall be checked regularly for undermining or deterioration and buildup or clogging with sediment. Corrective action shall be taken immediately.
- 4. All seeded areas will be checked regularly to see that a good stand is maintained. Areas should be fertilized and re-seeded as needed.
- 5. All temporary erosion and sediment measures shall be disposed of within thirty (30) days after final site stabilization is achieved and vegetation is established. Final site stabilization shall be approved by the City Inspector.

Permanent Stabilization

After final grade is achieved the site shall be permanently stabilized. Seed shall be applied to all grass areas per standard and specification provided on the plan sheets. Other areas will be characterized by impervious pavement or sidewalks, as shown on the layout plan, or enhanced with trees, shrubs, mulch, etc. As shown on the landscape plan.

Appendix A – USDA Soil Map / Geotechnical Report



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Lines



Soil Map Unit Points

Special Point Features

(o) Blowout



Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

Spoil Area

§ Stony Spot



Wet Spot

7

Other

.-

Special Line Features

Water Features

Streams and Canals

Transportation

+++

Rails

~

Interstate Highways

~

US Routes Major Roads

Local Roads

Background

The same

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Tidewater Cities Area, Virginia Survey Area Data: Version 21, Sep 5, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 9, 2022—Aug 15, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
24	Tomotley-Urban land complex, 0 to 2 percent slopes	0.3	100.0%
Totals for Area of Interest		0.3	100.0%

Appendix B – Water Calculations

WATER METER SIZING CALCULATIONS





Project Name: Hamtpon Pump Station #107 Replacement

Timmons Group Project No. 48544

Date: 09/13/2024

Calculated By: Alec Rediger

Fixture	Fixture Value (FU)	No. Fixtures	Total FU
Toilet (tank)	4.0		
Toilet (flush valve)	35.0		
Urinal (wall or stall)	16.0		
Urinal (flush valve)	35.0		
Bidet	2.0		
Shower (single head)	2.5		
Faucet (lavatory)	1.5		
Faucet (kitchen sink)	2.2		
Faucet (utility sink)	4.0		
Dishwasher	2.0		
Bathtub	8.0		
Clothes Washer	6.0		
Hose Connections (with 50' of hose)			
1/2 inch	5.0	1	5.0
5/8 inch	9.0		
3/4 inch	12.0		
Drinking Fountain	2.0		

Data Input		Notes and Descriptions
TOTAL FIXTURE COUNT	5 FU	Total of above Fixture Units
PROPOSED USE	Municipal	
PEAK DEMAND	10 GPM	From Figure 4-1 or 4-2 (M22, 2nd edition - 2004)

Table 4-1 (M22, 2nd Edition - 2004): Pressure Adjustment Values		
Pressure (PSI)	Adjustment Factor	
40	0.80	
50	0.90	
60	1.00	
70	1.09	
80	1.17	
90	1.25	
100	1.34	
Working Pressure	60 PSI	
Adjustment Factor	1.00	
Adjusted Peak Demand	10 GPM	
Design Peak Demand 10 G		

From Table 6-1 (AWWA Meter Standards)	
Meter Size	Max. Safe Flow
5/8"	20 GPM

WATER METER SIZING CALCULATIONS

CLUB HOUSE

PER AWWA-M22 METER DESIGN WORKSHEET



Project Name: Hampton Pump Station #107 Replacement Timmons Group Project No. 48544

Date: 09/13/2024

Calculated By: Alec Rediger

FIXTURE UNIT CONVERSION CURVES

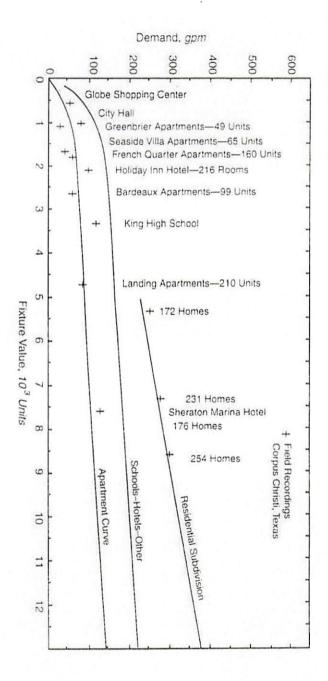


Figure 4-1 Peak flow demand of typical customer categories

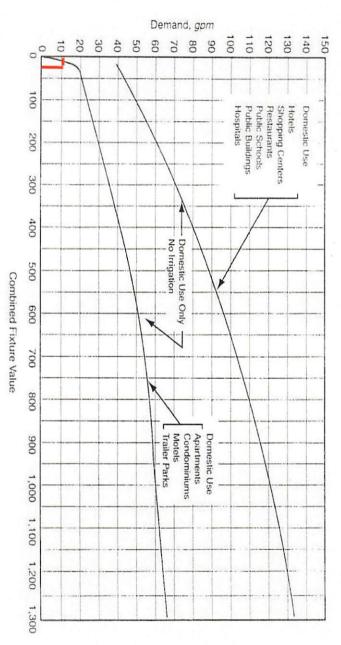


Figure 4-2 Water flow demand per fixture value low range

WATER METER SIZING CALCULATIONS

CLUB HOUSE

PER AWWA-M22 METER DESIGN WORKSHEET



Project Name: Hampton Pump Station #107 Replacement

Timmons Group Project No. 48544

Date: 09/13/2024

Calculated By: Alec Rediger

AWWA WATER METER STANDARDS (TABLE 6-1)

Meter	Minimum Flow Rate (gpm)	Low Normal Flow Rate (gpm)	Change-over Range (Compound Meters)	High Normal Flow Rate (gpm)	Maximum Flow Rate (gpm)	Head Loss at Maximum Flow (psi)
Positive 1	Displacement					
1/2 in	0.25		N/A	7.5	15	15
5/8 in.	0.25	1		10	20	15
3/4 in.	0.5	2		15	30	15
1 in.	0.75	3		25	50	15
11/2 in.	1.5	5	N/A	50	100	15
2 in.	2	8		80	160	15
Multijet						
5/8 in.	0.25	1	N/A	10	20	15
3/4 in.	0.5	2		15	30	15
1 in.	0.75	3		25	50	15
11/2 in.	1.5	5		50	100	15
2 in.	2.0	8		80	160	15
Turbine o	class 1					
3/4 in.	1.5	N/A	N/A	20	30	15
1 in.	2			35	50	15
11/2 in.	3			65	100	15
2 in.	4			100	160	15
3 in.	6			220	350	15
4 in.	8			420	630	15
6 in.	15			865	1,300	15
Turbine o	dass 2					
11/2 in.	4	N/A	N/A	80	120	7
2 in.	4	1.7774	14/11	100	160	7
3 in.	8			240	350	7
4 in.	15			420	630	7
6 in.	30			920	1,400	
8 in.	50			1,600	2,400	7 7
10 in.	75			2,500	3,800	7
12 in.	120			3,300	5,000	7
14 in.	150			5,200	7,500	7
16 in.	200			6,500	10,000	7
18 in.	250			8,500	12,500	7
20 in.	300			10,000	15,000	7
Compoun	d					
2 in.	0.25	2	20	80	160	20
3 in.	0.5	4	23	160	320	20
4 in.	0.75	6	28	250	500	20
6 in.	1.5	10	32	500	1,000	20
8 in.	2	16	50	800	1,600	20
Singlejet						
11/2 in.	0.5	1.5	N/A	50	100	15
2 in.	0.5	2.0		80	160	15
3 in.	0.5	2.5		160	320	15
4 in.	0.75	3.0		250	500	15
6 in.	1.5	4.0		500	1000	15

Source: Data are drawn from AWWA Standards C700, C701, C702, C708, C710, and C712, of latest revision as of December 2002.

N/A = not applicable



Project Name: Hampton PS 107 Replacement

Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

	WASTEWATER	FLOW GENERATION
Data Input		Notes and Descriptions
Facility Type	Various	Refer to Site Plans
Gross Square Footage		
Flow (GPD) per Unit	varies GPD/GSF	Refer to 2008 Hampton Roads Regional Technical Standards -
Flow Duration	varies HR	Sewage Flow Projection Data
Peak Factor	varies	
Average Flow	283 GPM	Unit Flow * Square Footage (converted from GPD to GPM)
Peak Flow	763 GPM	Average Flow * Peak Factor

	WET WE	ELL PARAMETERS		
Wet Well Geometry		Notes and Descriptions		
Wet Well Dimensions	14'x12'	Refer to Utility Plans & Lift Station Details		
Wet Well Interior Surface Area	168.00 SF			
Wet Well Elevations				
Top of Top Slab	8.50 FT	Refer to pump station plan		
Adjacent Ground	8.00 FT	Refer to grading plan		
Influent Pipe	-6.80 FT	Equal to invert of gravity influent pipe		
High Water Alarm	-7.80 FT			
Separation 0.50 FT				
Lag Pump On	-8.30 FT			
Separation 0.5 FT				
Lead Pump On	-8.80 FT			
Separation 2.3 FT				
Pump Off	-11.10 FT			
Separation 0.5 FT				
Low Water Alarm	-11.60 FT			
Separation 4 FT				
The state of the s				
Top of Bottom Slab	-15.60 FT			
Total Wet Well Depth	24.10 FT			
Depth from Adjacent Grade	23.60 FT			

STATIC HEAD			
Base Static Head		Notes and Descriptions	
Pump Off Elevation	-11.10 FT		
Force Main High Point	5.00 FT		
Base Static Head	16.10 FT	Difference between high point in FM and pump off elevation	
HRSD Force Main Pressures			
HRSD Pressure Head (Wet)	3.00 FT	Refer to attached HRSD Pressure Analysis Letter	
HRSD Pressure Head (Dry)	1.00 FT	Refer to attached HRSD Pressure Analysis Letter	
Total Static Head			
Total Static Head (Wet), H _{SW}	19.10 FT	Base Static Head + HRSD Wet Pressure Head	
Total Static Head (Dry), H _{SD}	17.10 FT	Base Static Head + HRSD Dry Pressure Head	



Project Name: Hampton PS 107 Replacement Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

120	Villiams C-Factor, C	M nəzsH	
± 006	quivalent Length, L	∃ lstoT	
12 FT	T = 0	A3 S	8" Gate Valve
24 FT	74 FT	A∃ r	8"x8" Tee (Branch)
18 FT	3 FT	A∃ 9	8. 42. Bend
TH 846 FT	THI	846 ∟F	8" Straight Pipe
геид _е р	геидth		(to HRSD Connection)
LefoT	Equivalent	Quantity	8" Force Main D = 8 in
120	Villiams C-Factor, C	W nəzsH	
102 FT	quivalent Length, L	∃ lstoT	
10 FT	10 FT	A∃ r	8"x6" Reducer
T = 0	E FT	A∃ 0	8"x8" Tee (Through)
19 FT	19 FT	A∃ ſ	8"x8" Tee (Branch)
32 FT	32 FT	A∃ r	8" Check Valve
3 FT	3 FT	A∃ r	8" Gate Valve
TH 0	T T	A∃ 0	8" 11.25° Bend
T = 0	3 FT	A∃ 0	8" 22.5° Bend
TH 0	3 FT	A∃ 0	8" 45° Bend
16 FT	T 8	A∃ S	8" 90° Elbow
T = 9	TH 1	9 LF	8" Straight Pipe (Through Valve Vault)
16 FT	1 FT	16 LF	8" Straight Pipe (Vertical in Dry Well)
геидұр	геидұр		发生的企业 的企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企业企
IstoT	Equivalent	Quantity	8" Force Main D = 8 in
The second second	RISTICS	AIN CHARACTE	FORCE M.

			W T3W - 3VRUS		
	Total Dynamic	Friction Head,	Friction Head,	Static Head,	Pump Flow,
	Head, TDH	H _f (8" FM)	H _f (6" FM)	waH	Ø
	(T ₁)	(TA)	(TH)	(TH)	(MAD)
	01.91	00.0	00.0	01.91	0
	04.61	72.0	60.0	01.61	100
	91.02	86.0	11.0	01.61	200
	21.40	70.2	62.0	01.61	300
- 3 _ 5 _ 6	23.02	3.52	04.0	01.61	001
11 1 13 13	25.02	5.32	09.0	01.91	009
	27.39	54.7	48.0	01.91	009
	51.05	16.9	21.1	01.61	004
Operation Point	32.26	28.11	1.34	01.61	044
per Pump Curve	33.22	12.68	44.1	01.61	008
	36.66	77.31	6Z.1	01.61	006
- 7	44.04	91.91	2.17	01.91	1000
	99.44	22.86	2.59	01.91	1100
	66.84	26.85	\$0.8	01.91	1200
	77.53	41.1E	3.53	01.91	1300
	98.83	17.35	90.4	01.91	1400

09.4

Friction Head, $\mathbf{H_f}$ Computed using the Hazen-Williams Formula: $H_f=10.44\frac{Q^{1.85}D^{4.8655}}{C^{1.85}D^{4.8655}}*L$

01.61

1200

75.04

72.48



Project Name: Hampton PS 107 Replacement

Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

			CURVE - DRY W		
	Total Dynamic	Friction Head,	Friction Head,	Static Head,	Pump Flow,
	Head, TDH	H _f (8" FM)	H _f (8" FM)	H _{SD}	Q
_	(FT)	(FT)	(FT)	(FT)	(GPM)
	17.10	0.00	0.00	17.10	0
7	17.18	0.08	0.01	17.10	50
	17.40	0.27	0.03	17.10	100
- 1	17.74	0.57	0.06	17.10	150
	18.19	0.98	0.11	17.10	200
1	18.74	1.47	0.17	17.10	250
	19.40	2.07	0.23	17.10	300
	20.16	2.75	0.31	17.10	350
	21.02	3.52	0.40	17.10	400
	21.97	4.37	0.50	17.10	450
	23.02	5.32	0.60	17.10	500
	24.16	6.34	0.72	17.10	550
	25.39	7.45	0.84	17.10	600
	26.72	8.64	0.98	17.10	650
	28.13	9.91	1.12	17.10	700
Operation Poir	29.63	11.25	1.28	17.10	750
per Pump Curv	31.22	12.68	1.44	17.10	800
	32.90	14.19	1.61	17.10	850
	34.66	15.77	1.79	17.10	900
-1 -1 a 1-1	36.50	17.43	1.98	17.10	950
	38.44	19.16	2.17	17.10	1000
	40.45	20.97	2.38	17.10	1050
	42.55	22.86	2.59	17.10	1100
	44.73	24.82	2.81	17.10	1150
	46.99	26.85	3.04	17.10	1200
	49.34	28.96	3.28	17.10	1250
	51.77	31.14	3.53	17.10	1300
	54.27	33.39	3.78	17.10	1350
	56.86	35.71	4.05	17.10	1400
	59.53	38.11	4.32	17.10	1450
	62.27	40.57	4.60	17.10	1500
	65.10	43.11	4.89	17.10	1550

Friction Head, $\mathbf{H_f}$ Computed using the Hazen-Williams Formula: $H_f = 10.44 \frac{Q^{1.85}}{C^{1.85} D^{4.8655}} * L$



Project Name: Hampton PS 107 Replacement

Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

	PUMP SELECT	TION
Pump Manufacturer	Fairbanks Morse	
Model Number	5" 5433 WD	
Motor Power	10 HP	
Speed	1185 RPM	
Voltage	208 V	
Phase	3-Phase	
Frequency	60 Hz	
Discharge Size	6-inch	
Impeller Diameter	10.16"	

		CYCL	E TIMES			THE RESIDENCE OF THE PARTY OF T
Lead Pump On Elev.	-8.80	FT		Force Main Dia	meter	6 IN
Pump Off Elev.	-11.10	FT		Cross-Sectiona	l Area 0.2	0 SF
Operating Height	2.30	FT				
Operating Volume	386.40	CF				
	2890.27	GAL				
		Wet Weathe	er - High Hea	d		
Flow Condition	Inflow Rate (GPM)	Pump Rate per Curve (GPM)	Fill Time (MIN)	Pump Run Time (MIN)	Cycle Time (MIN)	Velocity in Force Main (FPS)
Average Flow	283	750	10.2	6.2	16.4	8.51
Minimum Flow (Avg/2)	142	750	20.4	4.7	25.2	8.51
1/2 Pump Rate	375	750	7.7	7.7	15.4	8.51
Peak Flow	763	750	3.8	-222.3	-218.5	8.51
	Dry We	ather - Low He	ead (Runout	Condition)		
Flow Condition	Inflow Rate	Pump Rate	Fill Time	Pump Run	Cycle	Velocity in
		per Curve		Time	Time	Force Main
	(GPM)	(GPM)	(MIN)	(MIN)	(MIN)	(FPS)
Average Flow	283	800	10.2	5.6	15.8	9.08
Minimum Flow (Avg/2)	142	800	20.4	4.4	24.8	9.08
1/2 Pump Rate*	763	800	3.8	78.1	81.9	9.08
Peak Flow	763	800	3.8	78.1	81.9	9.08

^{*} In this scenario, half of the pump rate would exceed the incoming peak flow. The peak flow is substituted instead.

$$Fill\ Time = \frac{Operating\ Volume}{Inflow\ Rate}$$

$$\text{Pump Run Time} = \frac{\text{Operating Volume}}{\text{Pump Rate} - \text{Inflow Rate}}$$

Force Main Velocity =
$$\frac{\text{Flow}}{\text{Area}} * \left[\frac{1 \text{ CF}}{7.48 \text{ gal}} \right] \left[\frac{1 \text{ min}}{60 \text{ sec}} \right]$$



Project Name: Hampton PS 107 Replacement

Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

RESPONSE TIME	
Total Available Storage	12074 GAL
Peak Flow	763 GPM
Response Time	16 MIN
	0.3 HOUR

$$Response\ Time = \frac{Total\ Available\ Storage}{Peak\ Flow}$$

WET WELL STORAGE		
Wet Well Diameter	14'x12' FT	
Horizontal Area	168.00 SF	
High Water Alarm Elev.	-7.80 FT	
Wet Well Rim Elev.	8.50 FT	
Height Above Alarm	16.30 FT	
Storage Above Alarm	2738.40 CF	
	20483 GAL	

Wet Well Storage = Height Above Alarm * Horiz. Area

MANHOLE SS2 STORA	AGE
Manhole Diameter	5.00 FT
Horizontal Area	19.63 SF
Invert Out Elev.	45.26 FT
Manhole Rim Elev.	57.16 FT
Wet Well Rim Elev.	8.50 FT
Height to Lowest Rim	-36.76 FT
Available Storage	-721.78 CF
	-5399 GAL

Manhole Storage = Available Height * Horiz. Area

MANHOLE SS3 STORA	(GE
Manhole Diameter	4.00 FT
Horizontal Area	12.57 SF
Invert Out Elev.	44.66 FT
Manhole Rim Elev.	55.58 FT
Wet Well Rim Elev.	8.50 FT
Height to Lowest Rim	-36.16 FT
Available Storage	-454.40 CF
	-3399 GAL

Manhole Storage = Available Height * Horiz. Area

SEWER PIPE STORAGE	
Pipe Diameter	8.00 IN
Cross-Sectional Area	0.35 SF
Total Length	149 LF
Available Volume	52.01 CF
	389 GAL

Pipe Storage = Length * Cross Sectional Area



Project Name: Hampton PS 107 Replacement

Timmons Group Project No. 48544

Date: 08/22/2024

Calculated By: Dan Ruby

and the second of the second o	
lax Flow (Q)	800.00 gpm
ell Diameter	11.00 inches
ubmergence Required	38.83 inches
	3.24 ft
uction above slab	3.3 inches
ubmergence Provided	4.22 ft
Pump off to pump intake)	PASS

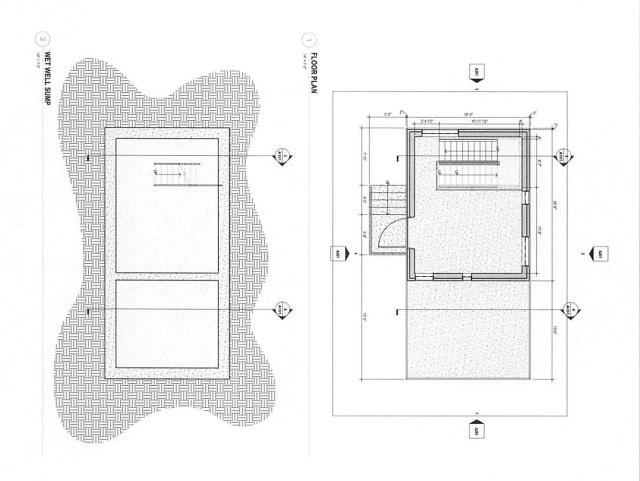
S = D + (0.574Q/D1.5)

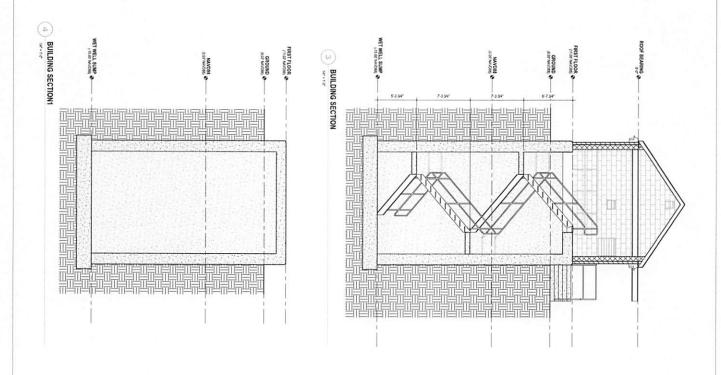
S = Req's min. Submergence in inches

D = bell diameter in inches

Q = Flow in gpm

Appendix C – Sewer Calculations





A101 FLOOR PLAN JOB NUMBER 2411 ISSUE DATE 09.17.24

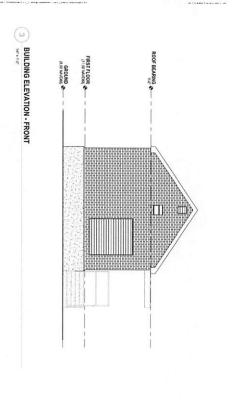
Hampton Pump Station 107

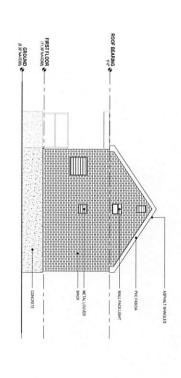
500 MICHIGAN DRIVE, HAMPTON VA 23669

Project Status

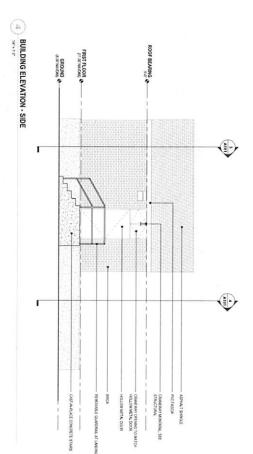


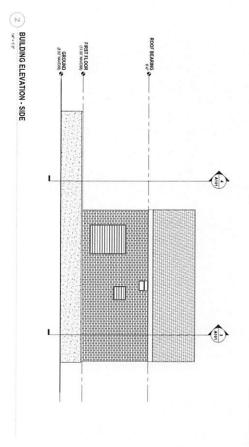
archsmith studio





BUILDING ELEVATION - REAR





CITY OF HAMPTON PUMP STATION #107 REPLACEMENT

CITY OF HAMPTON, VIRGINIA **CIVIL SITE PLANS**

SITE PLAN NO. SP2023-0013 SEPTEMBER 13, 2024

PROJECT INFORMATION:

OWNER HAMPTON REDEVELOPMENT AND HOUSING AUTHORITY

PARCEL INFORMATION LASALLE AVE

LRSN. 2000401

BUILDING FLOOR SPACE

STORMWATER DESIGN CRITERIA

HUC CODE

WATERSHED LYNNHAVEN-POQUOSON

ONSITE RESOURCE PROTECTION AREA

TOTAL PARCEL AREA 14.579 SF (0.34 AC) DISTURBED AREA PROPOSED PS SITE: DISTURBED AREA EXIST. PS SITE: DISTURBED AREA IN ROW: 6.875 SF (0.16 AC)

EXISTING DEVELOPMENT IMPERVIOUS AREA (WITHIN LIMITS OF DISTURBANCE): PROPOSED DEVELOPMENT IMP. AREA WITHIN LIMITS OF DISTURBANCE)

VERTICAL DATUM NAVDes

VIRGINIA STATE PLANE SOUTH ZONE, NORTH AMERICAN DATUM 1983/2011 (US SURVEY FEET)

EXISTING USE PROPOSED LAND USE PUMP STATION METHOD OF REFUSE PRIVATE

SOIL TYPE(S) 24 | TOMOTLEY-URBAN LAND COMPLEX | HSG B/D | SLOPES 0 TO 2 PERCENT

REGULATORY DATA HAMPTON SPECIAL INTEREST DISTRICT NONE

Sheet List Table

Sheet Number	Sheet Title		
C0.0	COVER SHEET		
C0.1	GENERAL NOTES		
C1.0	EXISTING CONDITIONS		
C2.0	DEMOLITION AND EROSION & SEDIMENT CONTROL		
C3.0	SITE LAYOUT PLAN		
C4.0	GRADING PLAN		
C5.0	UTILITY PLAN		
C5.1	UTILITY PROFILES		
C5.2	UTILITY PROFILES		
C5.3	UTILITY PROFILES		
C6.0	SITE DETAILS		
C7.0	UTILITY DETAILS		
C7.1	UTILITY DETAILS		
C7.2	UTILITY DETAILS		



VICINITY MAP

OWNER: TRILOGY DEVELOPERS, LLC ROSS VIERRA 1439 N GREAT NECK RD VIRGINIA BEACH, VA 23454
PHONE: (757) 470-2068
EMAIL: RVIERRA@AXISGE.COM

SURVEYOR:
TIMMONIS CRICUP
11527 ROCK LANDING DRIVE, SUITE 306
NEWPORT NEWS, VIRGINIA 23006
PHONE: (757) 723–304
FAX: (757) 723–3054
CONTACT: ROB MANN LIS
EMAIL: ROB MANNESTIMMONS COM

TIMMONS GROUP
2001S. LYNNHAVEN ROAD, SUITE 200
VRGINIA BEACH, VIRGINIA 23452
PHONE: (757) 213-0601
FAX: (757) 360-1415
CONTACT. DAN RUBY, P.E.
EMAIL: DAN RUBY @TIMMONS.COM

NEWPORT NEWS WATER WORKS SUITE 400 NEWPORT NEWS, VIRGINIA 23606

HAMPTON ROADS

SANITATION DISTRICT 2401 G AVENUE NEWPORT NEWS, VIRGINIA 23602 P. (757) 833-1729 PUBLIC WORKS CITY OF HAMPTON

HAMPTON, VIRGINIA 23669 P. (757) 727-8311 RODNEY KIEFERT: (757) 726-2984

WATER SERVICE DATA PER AWWA-M22 METER DESIGN WORKSHEET

10 GPM 0.625*

PEAK DOMESTIC DEMAND DOMESTIC METER SIZE: DOMESTIC SERVICE LINE SIZE:

NOTE: ALL ON-SITE UNDERGROUND UTILITIES MUST BE IN ACCORDANCE WITH THE LATEST EDITION OF THE CITY OF HAMPTON PUBLIC WORKS DESIGN AND CONSTRUCTION STANDARDS AND CHAPTER 9 OF THE CITY CODE (REF. CITY CODE SECTION 35.1-43)

DEVELOPER: TRILOGY DEVELOPERS, LLC ROSS VIERRA 1439 N GREAT NECK RD

ARCHITECT:
COX RUENDER & COMPANY, P.C.
COX RUENDER & COMPANY, P.C.
COX1 VINGRIMA BEACH BLVD
VINGRIMA BEACH LIVE
VINGRIMA BEACH
CONTACT:
GREYMASON
EMAL. GREYMA

PUBLIC WORKS CITY OF HAMPTON ENGINEERING SERVICES 22 LINCOLN STREET HAMPTON, VIRGINIA 23669 P. (757) 727-6346 F. (757) 727-6123

WASTEWATER OPERATIONS

MISS UTILITY OF VIRGINIA

THIMMONS GROUP IS NOT RESPONSIBLE OR LIABLE FOR ANY CONSTRUCTION OR DAMAGES TO THIS PROJECT PRIOR TO ALL FINAL PLAN APPROVALS.

A CONSTRUCTION GENERAL PERMIT (CGP) IS REQUIRED PRIOR TO OBTAINING A LAND DISTURBANCE PERMIT FROM THE CITY OF HAMPTON.

requirements and the City of Hampton's property plat approval process.

RESPONSIBLE LAND DISTURBER NOTE

All property lines labeled as To Be Established shall be duly recorded at the Circuit

Court of Hampton prior to City of Hampton issuance of a building permit. Contact the City of Hampton Development Services Center [757-727-2444] for information pertaining to the subdivision / property line vacation / boundary line adjustment plat

RESPONSIBLE TO THE PROFESSIONAL WHOSE SEAL IS AFFOCD HEREON SHALL ACT AS THE TRESPONSIBLE LAND
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RESPONSIBLE LAND DISTURBER DESIGNATION

IBLE LAND DISTURBER CERTIFICATE IFICATION FOR COMBINED ADMINISTRATOR, ADMINISTRATOR, PLAN REVIEWER, R. OR CONTRACTOR. ROY ESSIONAL ENCINEER, LAND SURVEYOR, LANDSCAPE ARCHITECT, OR ARCHITECT

RESPONSIBLE LAND DISTURBER CONTACT INFORMATION: NAME (PRINT)

MAILING ADDRESS TELEPHONE

Approved by the City of Hampton Date

DIRECTOR OF PUBLIC WORKS

COMMUNITY DEVELOPMENT DEPARTMENT, CITY AGENT

THE DRAWING PREPARED ATTH VIRGINIA BEACH OFFICE South Lymbrices Read, Suite 200 | Virginia 1 757-213 6679 FAX 757 340.1415 www

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09/13/202

A. REDIGE A. REDIGER CHECKED BY n puny

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REPLACEMENT

STATION 107

PUMP HAMPTON

48544 CO.0

SP2024-XXXX

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CHILLECTUD BE 18 FOR 1865 NEWSYLTS WOULD FOR THE CONTINUENCY OF THE CHILL OF THE CONTINUCTOR SHALL BE RESPONSEEL FOR REPLACING WITH NEW COMMECTIONS ARE TO BE MADE. DI DEPARTMENT OF PUBLIC WORKS DESIGN AND CONSTRUCT NE VAID SEWEN FORCE MAINS IN ACCO 2: SENT CONTINUES OF EXISTING SANCUT ASPINALT WITH APPROPRIATE SENLANT PROOF TO PLACEMENT OF NEW ASPINALT STANKS, AND PUBLIC SANTARY SEWER PACILITIES INCLUSIVE OF GRAVITY IN. THE CONTINCTOR SHALL BE RESPONSIBLE FOR PREPARING AS-BUILD STORES.

CHANNINGS FOR STORMWATER MANAGEMENT FACELITIES, PUBLIC STORES. MONITOWN COLUMN SWAFT RE PERSONANCO ON EDUCES OF PAYENERS AND CONCRETE. SUES CONSTRUCTION FIRMS AND PROPERTY. 8. CONTINUEDRISH AND WASTONING AND IS UNTILED TO THE ANGLAND THE. DEMOLITION NOTES: DOM:NAGO 11 IF A CONSTRUCTION TRALER CONTAINED FROM THE HEALTH OR A PURMED FROM THE HEALTH OR A PURMAL PRESENCE TO THE SEVER BYSTEM OR A PURMAL PRESENCE TO THE SEVER BYSTEM OR A PURMAL PROPERTY. OR REPLACED, SHALL BE DONE SO TO THE NEAREST JOHT AL CONCRETE SIDEWALKS, CURSS, AND GUTTERS INDICATED TO BE REMOVED SELOW FINISHED SUPPLICE, ROUND EDGES OF CONCRETE 1/9-INCH RADIUS, MLS SHYCED VI 50+001 MIERWAY? DOWNA TOWIS SHAFT BE HYPICAENZE DOWNA TOWAR VA PLOOF WITERVALS, 1/24NCH TRANS KAN TR IN ACTIVITIES. 2" ONLESS OTHERWISE NOTED ON THE PLANS, ALL SOEMALIS SHALL HAVE SECULORD TO DRAW, TOPSOLED, SEEDED, MULCI-ED, AND FERTILIZED 4 ALL UNPAYED ANGAS DISTURBED BY THE CONTRACTOR'S OPERATIONS SAME. ACCORDANCE WITH THE CITY OF HAMPFOW UTLITY POLICY. SMOLLONG' BLINDCLONGS' ON LICED ORGENOCLONGS IT EXEMPSION TORKLE IN CONCRETE SHAFT BE PROVIDED AROUND ALL CHIE CHIN LEDGISHT VCCE 22/08/LLL 21/MCMC)2 WHAT WARE AND BURES SWEET DE M WOODSMACE MILE NAMES APPROVED LIST OF CERTIFIED CONTRACTORS. 13 YET MYSEISTING DELIVER OF THE WEST BEST BETWEEN STANDARDS. THE CONTINUED STANDARDS THE WASTERSTHEN STANDARDS SALES ON THE 13 ALL MATERIALS SALES ON THE WASTERSTHEN SALES SALES ON THE SALES OSTON SSMRSHTO SE LISON LYCE OF BUILDING OR PACE OF CURB UNLESS COLON 13 THAL WATER DE CHECKH FOR MEGAS COCATED WITHIN THE ROUT-CF-WAY OR PREMAINS WHITH WENDORS WERE DECARRENTS SHALL BE FER THE PLANS PREMAINS SHALL BE FER THE PLANS WATER WAS WATERWORS SHALL BE FER THE PLANS WATER MED ON THE WORLD WATER WAS SHALL BE FER THE PREMAIN WHITH WE SHALL BE FER THE PLANS. 10. ALL RADII AND CURES MOICATED SHALL HAVE CONTINUOUS SMOOTH TRANSCITIONS WITHOUT ARRUPT CHANGES, BENDS, OR FACETED EDGE. ALL RADIAL RATION IN A SMOOTH CONTINUOUS MANNER, FLUSH ALONG ENTRE COMMON EDG VIT MODOSED BYANG SONG VOES SHYIT NEET VOTYCENT BYANG SONG VCES ELECTRICAL PLANS MAXMAN S DY CROSS SLOPE AND SHALL MEET ALL LOCAL AND FEDE COMMINST TO THE DRAWWAGE. VIT KAYING WID SEW WE SAKED STING YOSES SHATT HAKE Y ROBINING I DAY WID STRENGTH OF 2500 PSI WITH SN. TO I'M AM ENTRADMENT. IF WE'VE THE CENTERING OF THE PIPE PROJECTED ONTO THE HORZONT

THE PLAN PREETS AND BACK AND DUCLIFE BOWN BYE, BROYCE STATUOR

THE PLAN PREETS AND BACKED UPON ALLOWAGE DEFLECTIONS OF JOHNS 7. ON-SITE EXTERIOR CONCRETE IS TO HAVE A MINIMUM COMPRESSIVE N34013A30 3H CONTINUENTS DISTURBED DURING CONSTRUCTION AT NO ADDITIONAL COST TO THE CONTINACTOR SHALL LEE FIPE ANDORS JOHN DEFLECTION OAS SPECKED THE FIPE PREMANANCTURES OR RITHOSO AS INCRESANTS TO OBTIAN THE REQUIRED HORIZORIAL MAD VERTICAL ALCOMENT OF THE PROPOCED MATER AND SEWER LACES. THE CONTINUE ON SHALL RESERVACION AND EXISTING PROPERTY PINS AND TOSCOPIA SHIT TUCHOLOGISHI GAMARIMMAR (MATA SHORTADO) TANCONIC STAND OF DISTURBED SAME BY TANCE DESCRIPTION TO THE PRODUCTION. ALL DISTRIBUTION OF AVERAL, BHOLLOWING BUT NO ELEMENT.

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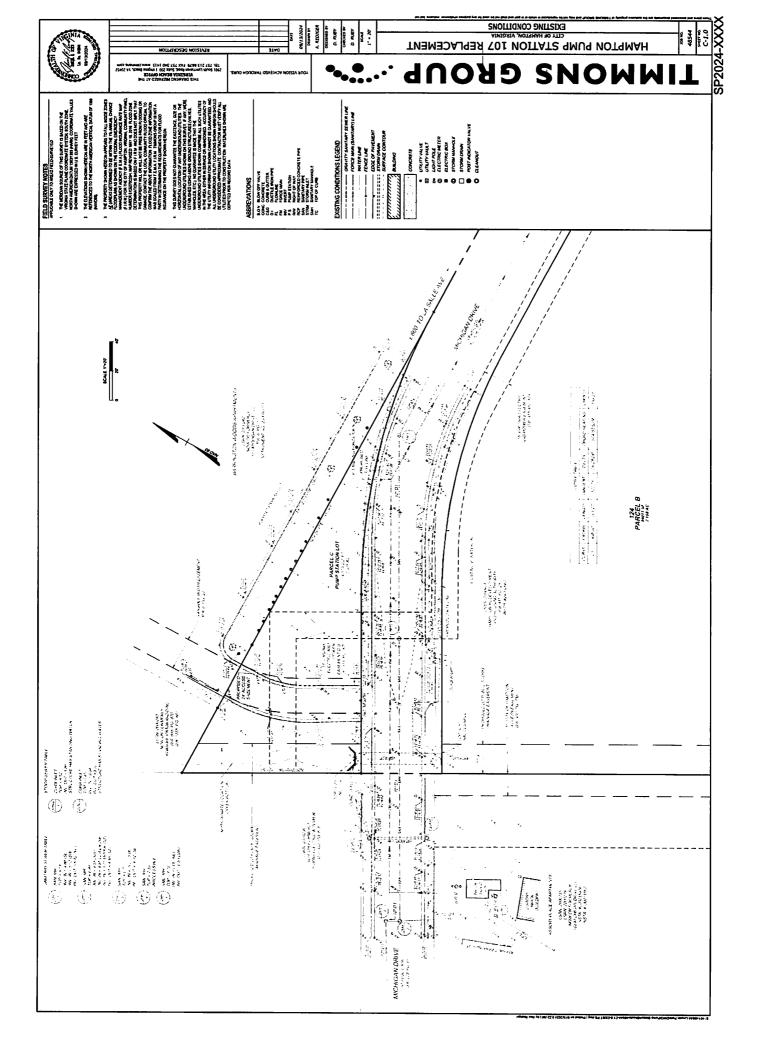
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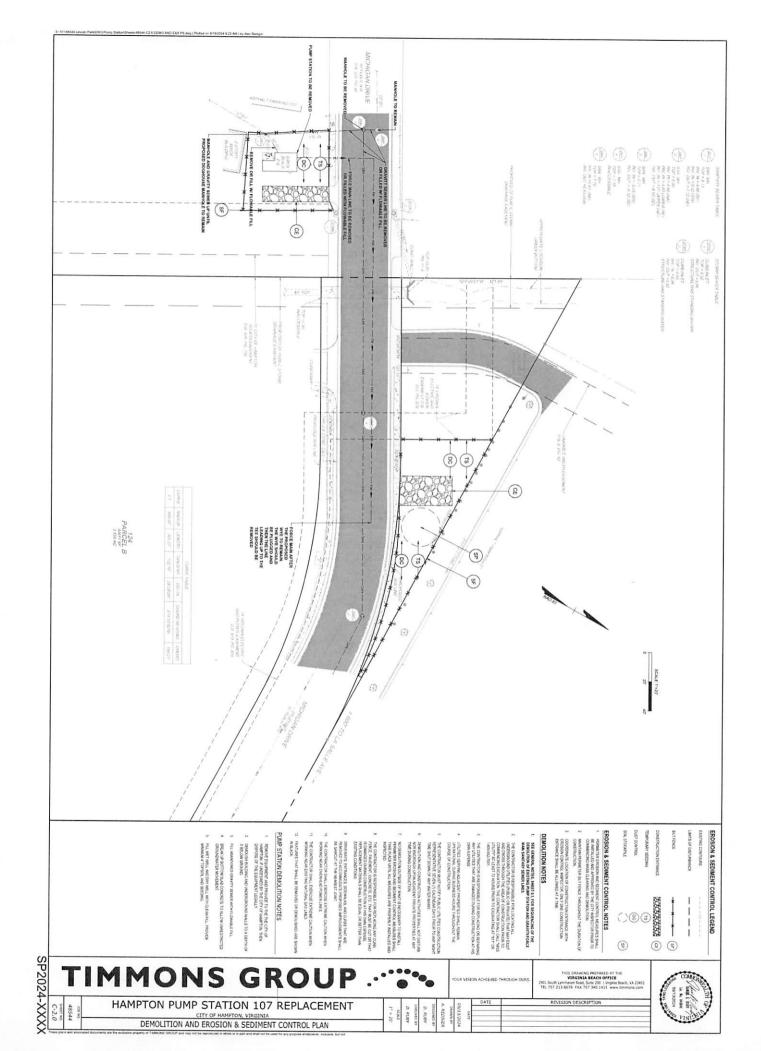
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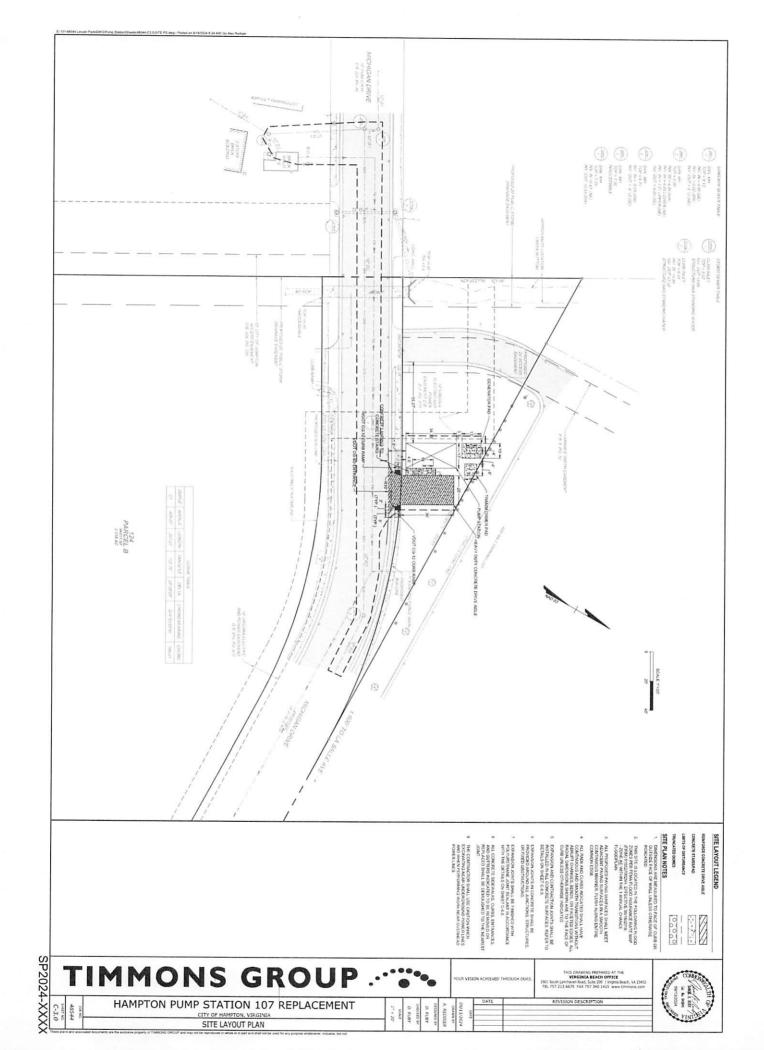
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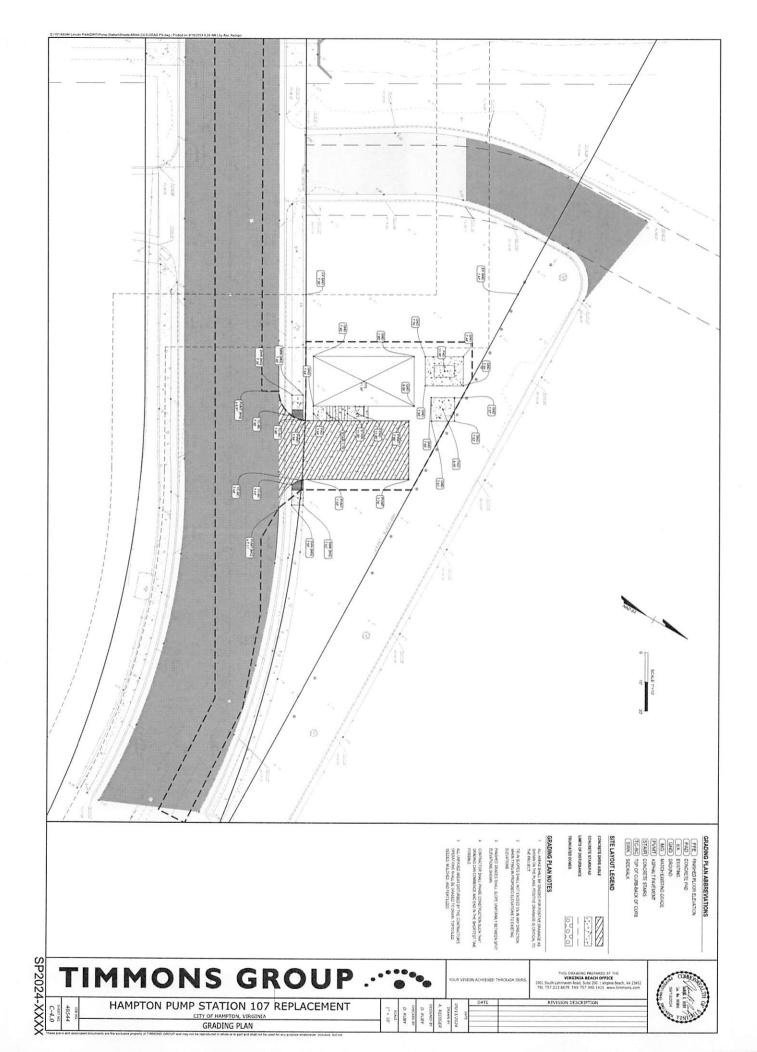
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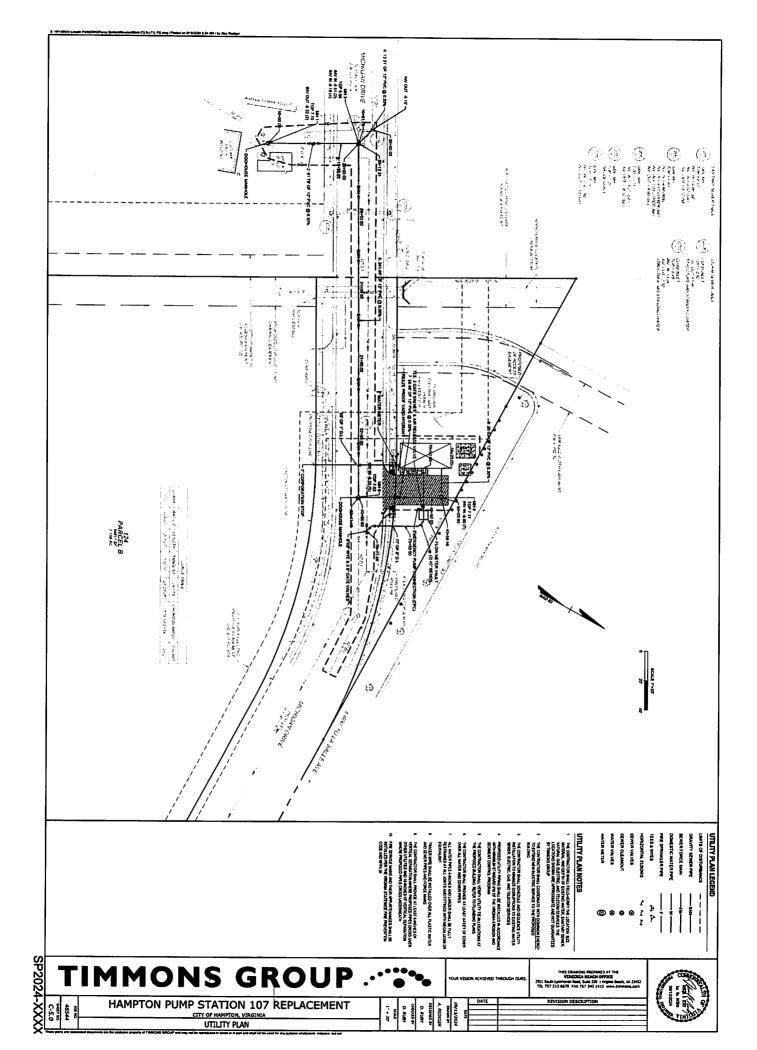
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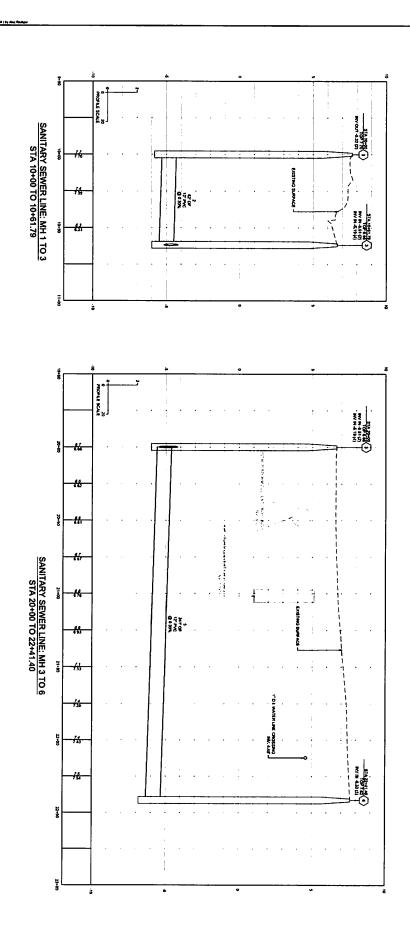


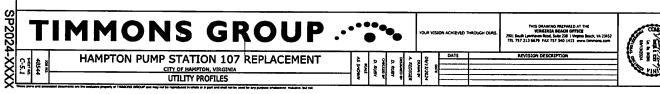


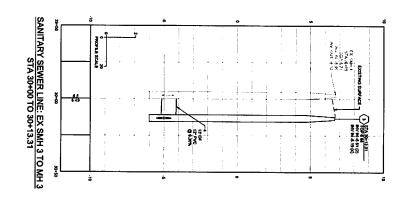


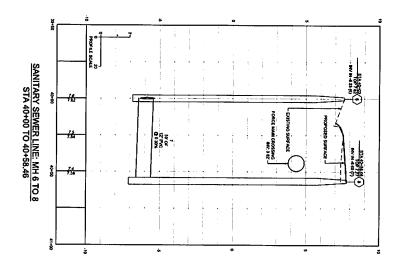






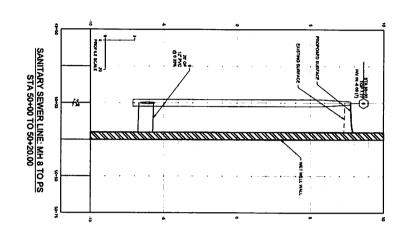


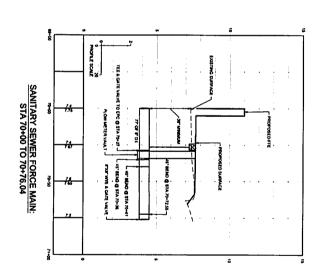




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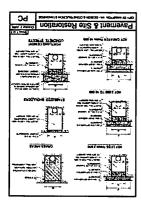
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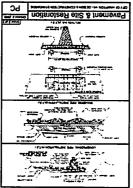
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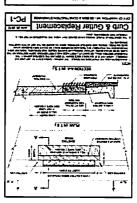
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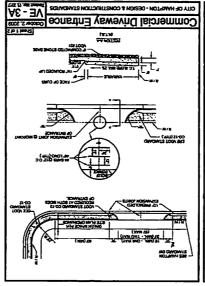
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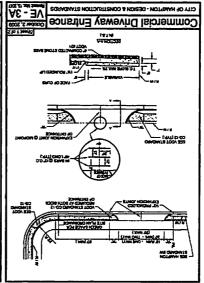


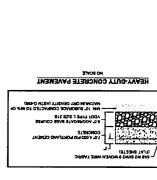
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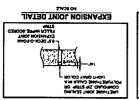


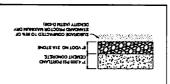


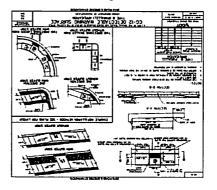


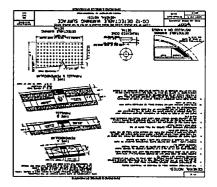


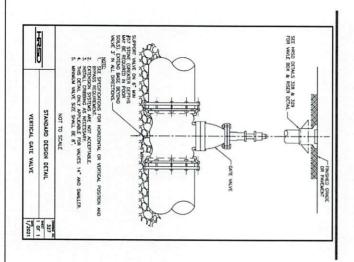
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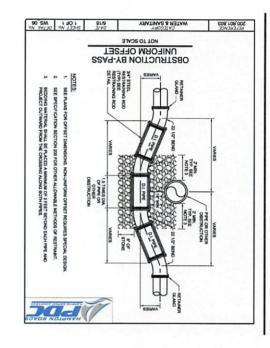


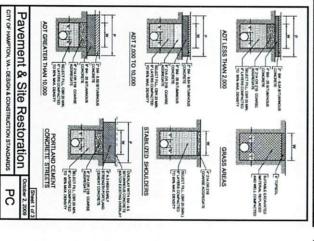


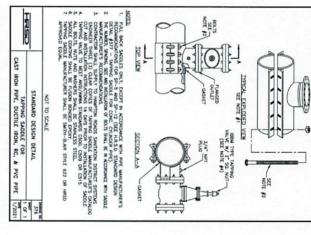


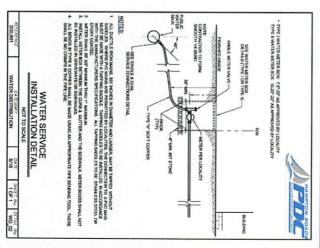


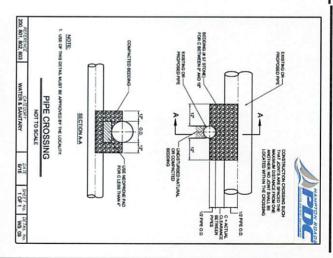














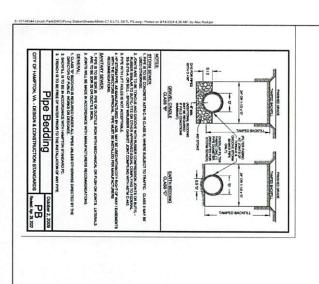
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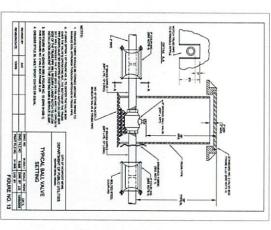
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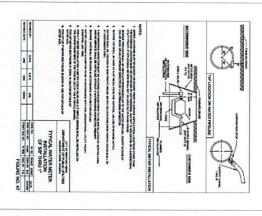
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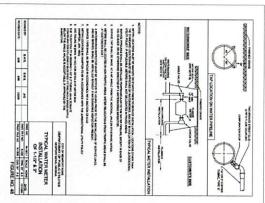
SP2024-XXXX

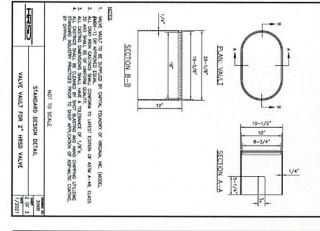
48544 SHET NO. C-7.0

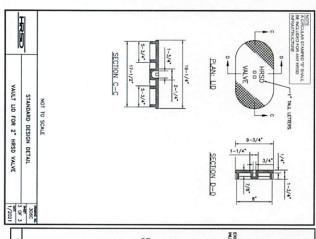


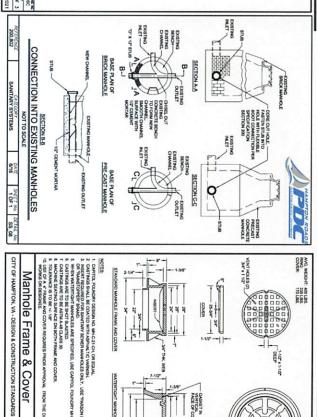


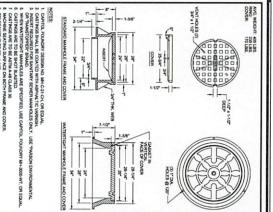














HAMPTON PUMP STATION 107 REPLACEMENT

UTILITY DETAILS

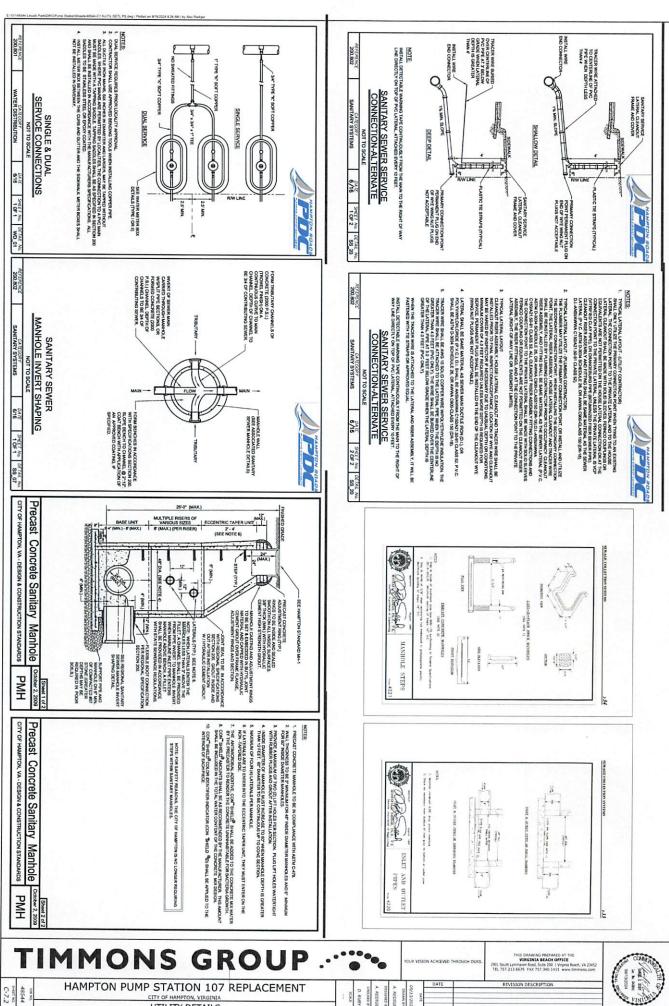
D. RUBY

SP2024-XXXX

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Manhole Frame & Cover



SP2024-XXXX

UTILITY DETAILS

D. RUBY

